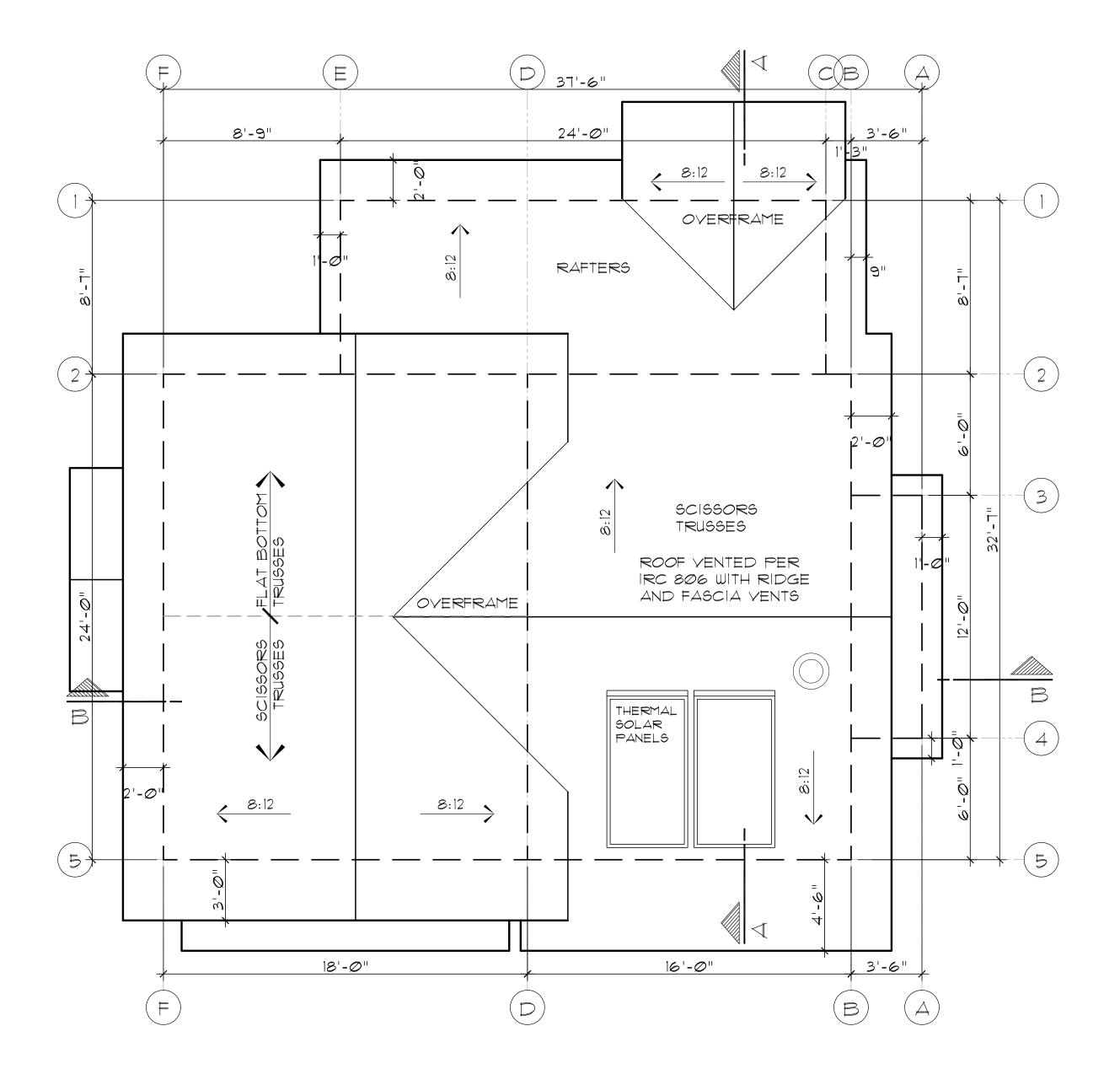


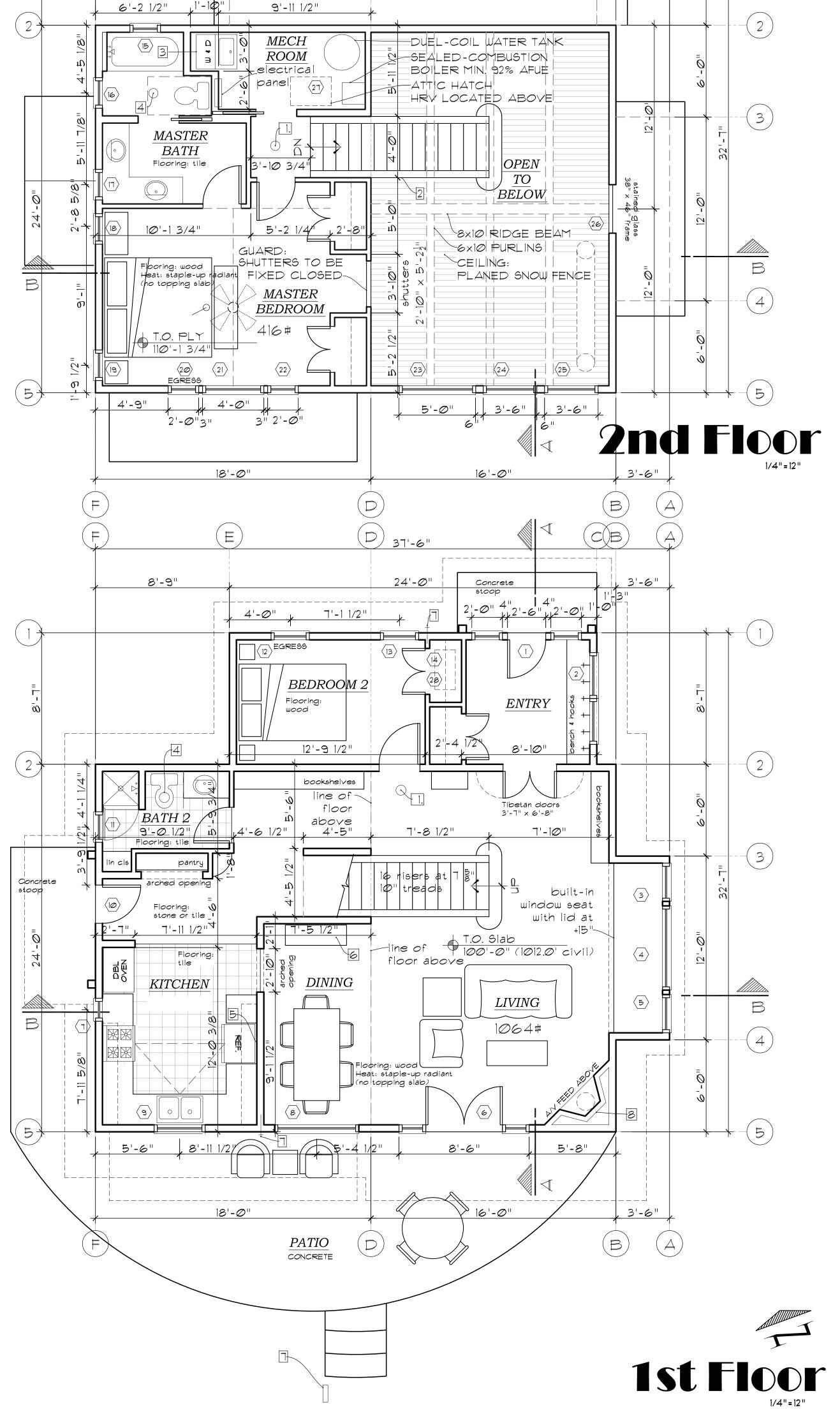
- Proposed house is in a moderate
- wildfire hazard area. Sunscreen is designed to be constructed of non-combustable
- materials. Roof min. class B
- Roof ventilation shall not be in soffit Porch roofs
- beams min 6x10 rafters min 4x6 min 6x6 columns mín 2x T‡G deckina Overhangs greater than 48"
- min 6x10 beams rafters min 4x6 braces min 6x6 decking min 2× T∉G

PLAN NOTES:

- 1. SMOKE ALARM, INTERCONNECT WITH ALL OTHER SMOKE ALARMS.
- HANDRAIL PER IRC R311.5.6
- 3. YENT DRYER TO OUTSIDE. 4. MECHANICAL VENTILATION FAN
- (OPTIONAL)
- 5. VENT RANGE TO OUTSIDE (OPTIONAL) 6. PASSIVE AIR GRILL
- FROST-PROOF HOSE BIB
- 8. WOOD BURNING FIREPLACE, MAKE \$ MODEL TO BE DETERMINED, MAKE & MODEL TO BE SELECTED FROM ECO'S LIST OF APPROVED DEVICES.



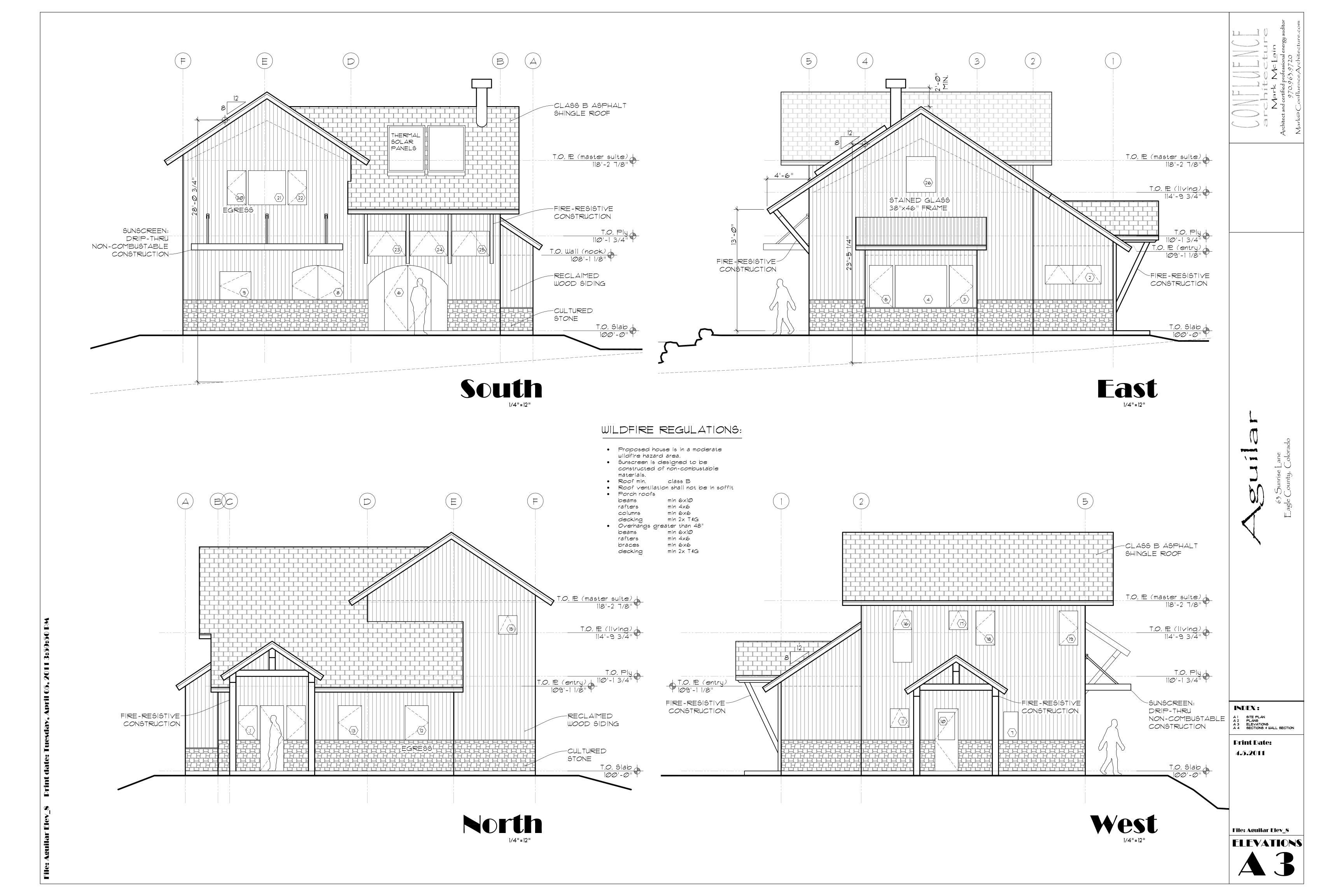


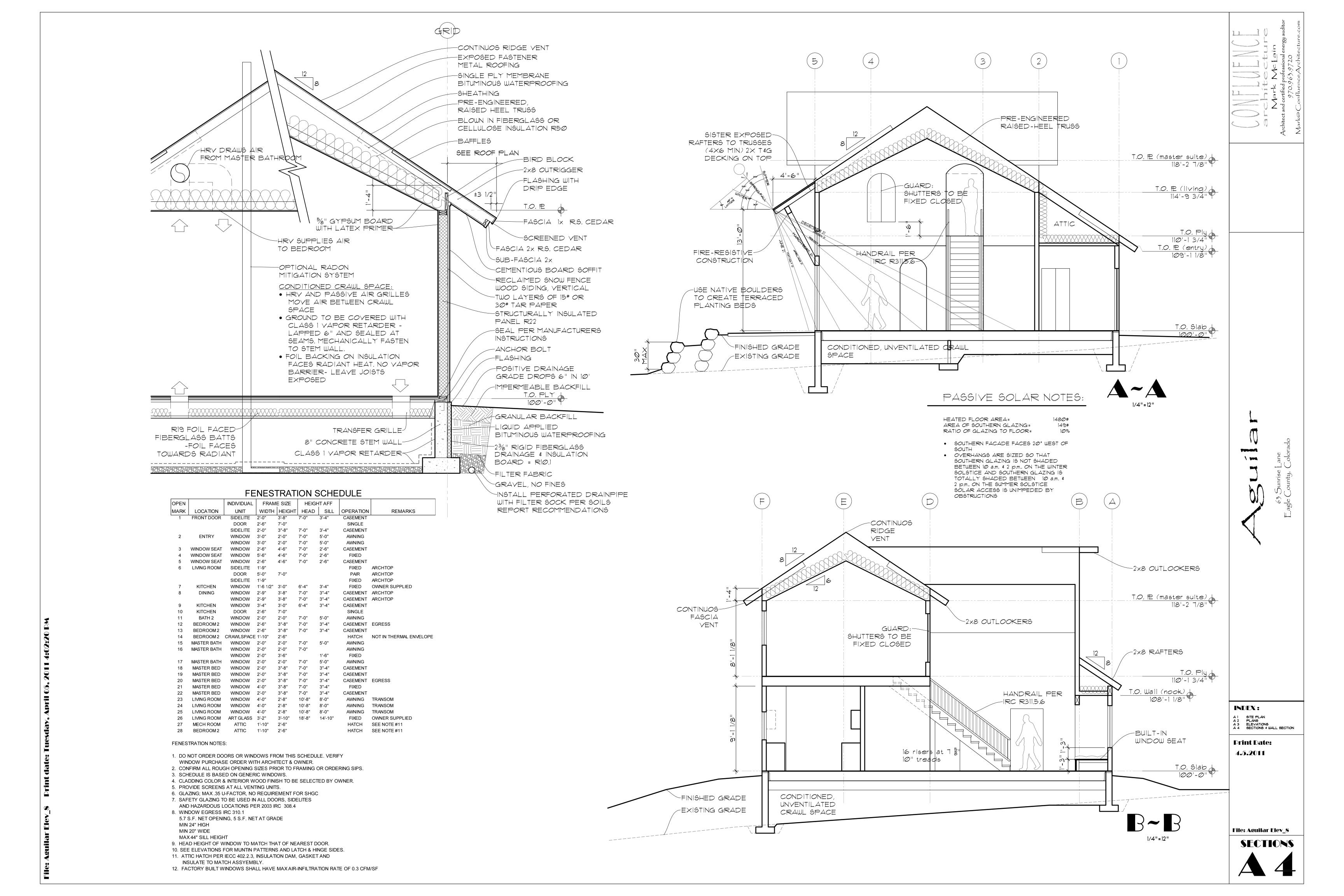


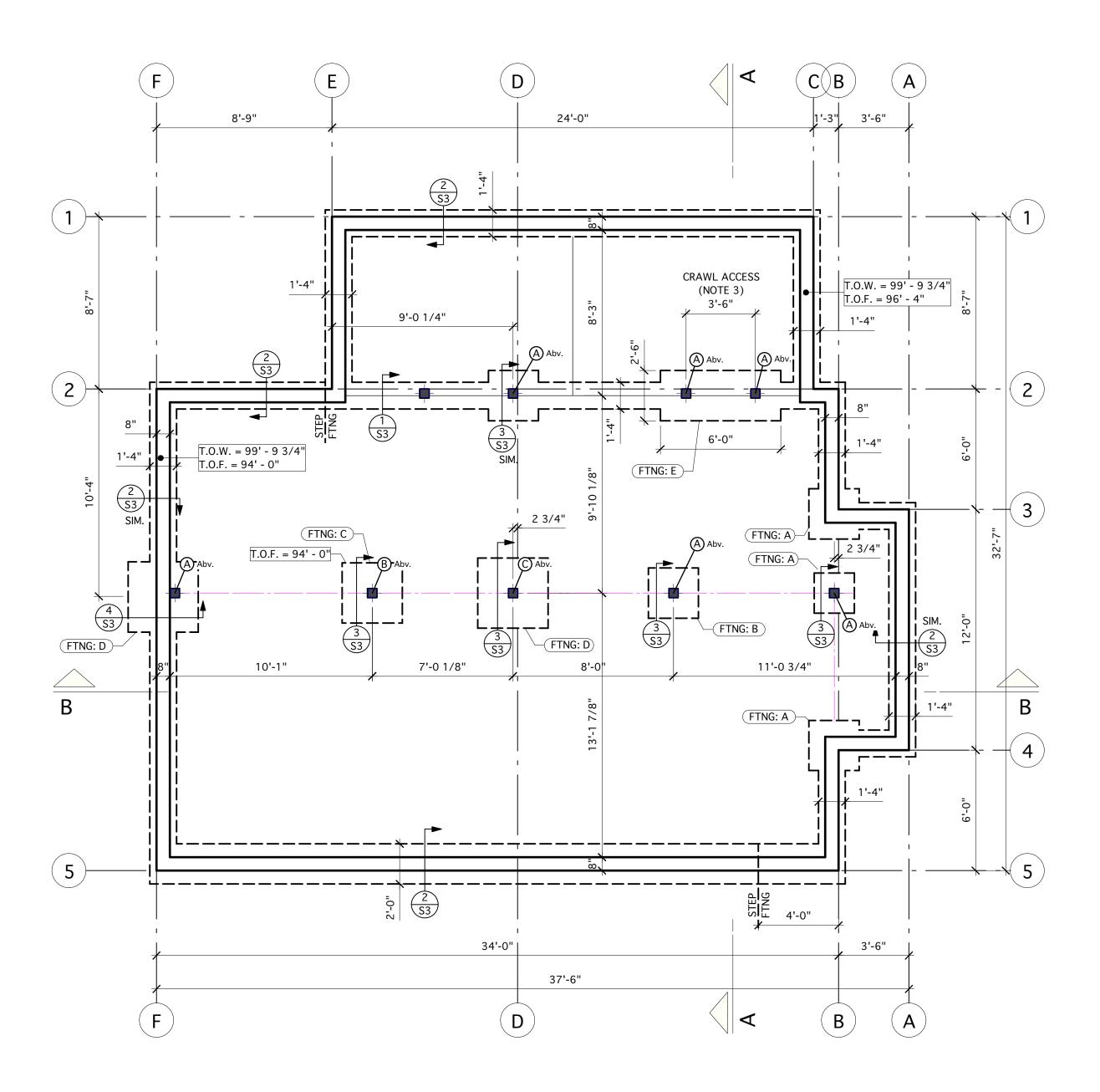
INDEX: A 1 SITE PLAN
A 2 PLANS
A 3 ELEVATIONS
A 4 SECTIONS & WALL SECTION Print Date: 4.5.2011

File: **A**guilar Plan_8

PLANS







FOOTING SCHEDULE						
DESIG.	SIZE	THICK.	REINFORCING, EA. WAY			
FTNG: A	2'-0" SQ.	10"	#4 @ 8" O.C.			
FTNG: B	2'-6" SQ.	10"	#4 @ 11" O.C.			
FTNG: C	3'-0" SQ.	10"	#5 @ 14" O.C.			
FTNG: D	3'-6" SQ.	10"	#5 @ 12" O.C.			
(FTNG: E	SEE PLAN	10"	#5 @ 12" O.C.			

- NOTES
 Footings shall bear on undisturbed natural soil and cobble soils. All fill,topsoil, clay and loose disturbed soils shall be removed to the level of naturally occuring gravel and cobble soils.
 Field verify footing step locations to maintain adequate frost protection (48" in Eagle County) and to bear on existing soils.
 Field locate crawlspace access (3'-0" x 3'-0" MAX). Do not locate at point load above

- above.

F		D		B A	
		C. C	1.88 8.1) Abv.	1
2		Ada	-) Aþv.	(2)
	(A) Abv.	2 - 11 7/8" LVL [99'11 1/4"]		A Abv.	(3)
-	2 - 11 7/8" LVL [98'11 3/8"]	A Abv. B Abv. 2 - 11 7/8" LVL Blw.	2 - 11 7/8" LVL [98'11 3/8"]	### T.M. 210 @ 76" 0.C.	
B	BBIW. T.O.PLY 100' -		@ 16" 0.C.	A Abv. A Abv.	B - 4
5	AAAbv. AAbv. AAbv. AAbv.		(Cout. 1782.06/11.88	A Abv.	(5)
		A Abv.			3
F		D		B	

COLUMN LEGEND

- 1. DENOTES COLUMN BELOW.
- 2. DENOTES COLUMN ABOVE OR TRANSFERED THIS LEVEL.
- 3. ☑ DENOTES CONTINUOUS COLUMN.

(H) - 8 x 8 POST

- 4. K.P. = KING POST 5. D.T. = DOUBLE TRIMMERS
- 6. PROVIDE ENOUGH STUDS TO MATCH WIDTH OF BEAM ABOVE. 7. ALL COLUMNS SHOWN ARE 2-2x6's UNLESS NOTED AS FOLLOWS:

WOO	<u>DD</u>		PARA	۱LL	AM, PSL	STEE	L	
A	-	3-2x6	J	-	31/2 x 31/2	<u>(S)</u>	-	HSS 31/2 x 31/2 x 1/4
$^{\circ}$	-	4-2x6	(K)	-	31/2 x 51/4	\bigcirc	-	HSS 4 x 4 x 1/4
©	-	5-2x6	L	-	51/4 x 51/4	<u>(U)</u>	-	HSS 4 x 4 x 3/8
D	-	3-2x4	\bigcirc	-	51/4 x 7	\bigcirc	-	HSS 4 x 4 x 1/2
(E)	-	4-2x4				\bigcirc	-	HSS 5 x 5 x 1/4
F	-	5-2x4				Y	-	HSS 5 x 5 x 1/2
G	-	6 x 6 POST				\bigcirc	-	HSS 6 x 6 x 5/16

P - HSS 7 x 3 x 5/16

REVISIONS

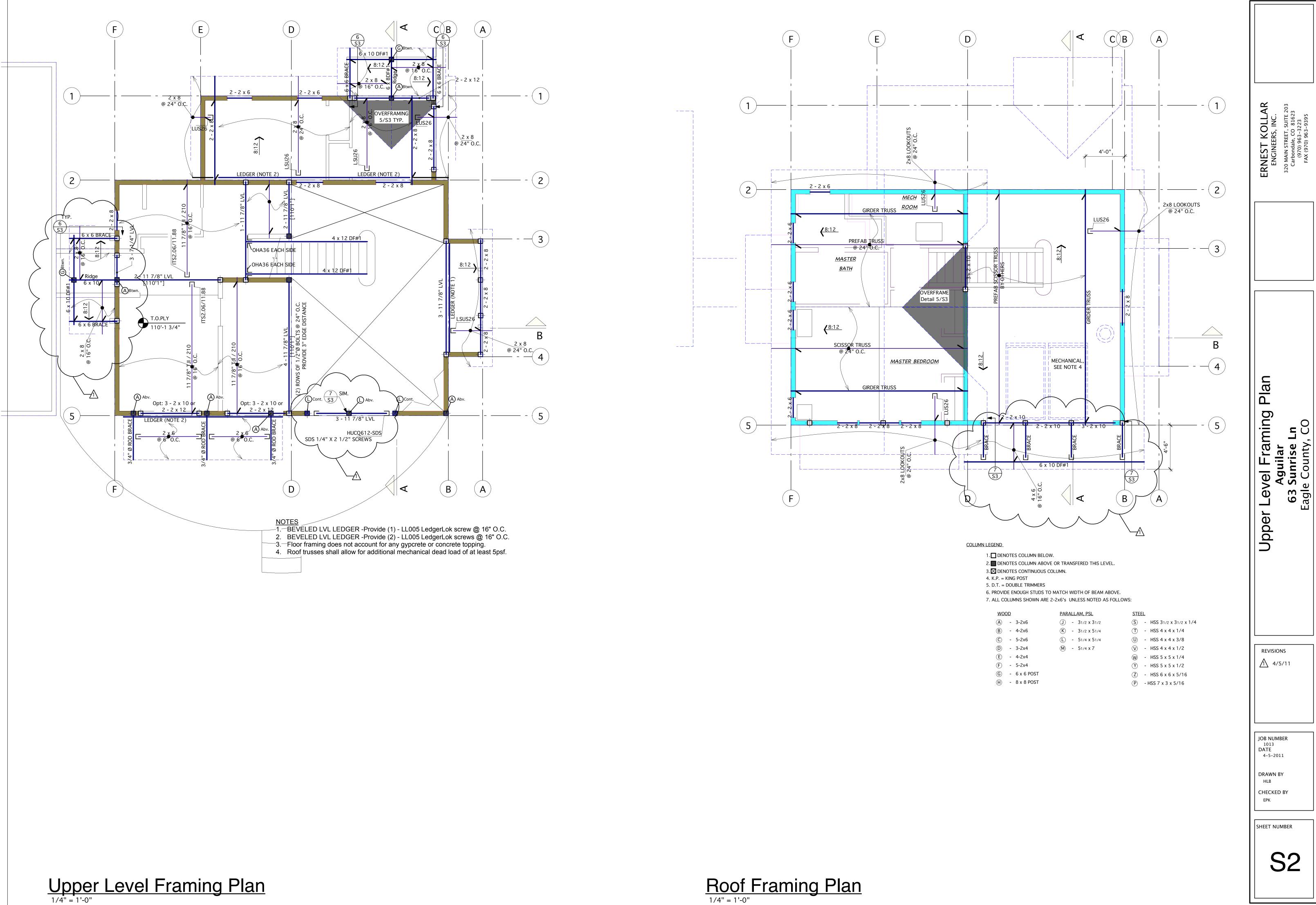
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JOB NUMBER 1013 DATE 12-13-2010

DRAWN BY HLB CHECKED BY

SHEET NUMBER



Roof Framing Plan

1/4" = 1'-0"

4-5-2011 DRAWN BY

SHEET NUMBER

STRUCTURAL GENERAL NOTES & SPECIFICATIONS

DESIGN DATA:

- A. DESIGN LIVE LOADS 1. Basic Roof Snow 4. Garage 40 PSF 2. Residential Floors 5. Wind 90 mph, Exposure B 52 PSF Decks Seismic Zone C B. DESIGN CODES
- International Residential Code, (IRC), 2003
- with the International Building Code, (IBC), 2003 for Special Inspection American Concrete Institute Building Code (ACI 318-99)
- American Institute of Steel Construction, Manual of Steel Construction (AISC 9th Edition) American Institute of Timber Construction, Timber Construction Manual (AITC 5th Edition)
- National Design Specifications for Wood Construction (NDS2001)

GENERAL:

- 1. Any changes to the contract drawings shall be submitted to the structural engineer for approval.
- 2. All dimensions and details on structural drawings shall be verified against the architectural drawings and any discrepancies shall be brought to the attention of the architect prior to fabrication or construction. Details on the structural drawings are typical. DO NOT SCALE
- 3. All openings through the structural system shall be approved by the Engineer.
- 4. Contractor shall provide all necessary temporary bracing, shoring, guying or other means to prevent excessive stresses and to hold the structural elements in place during construction.
- 5. The contract structural drawings and specifications represent the finished structure. They do not indicate the method of construction. The contractor shall provide all measures necessary to protect the structure during construction. Such measures shall include, but are not limited to bracing, shoring for loads due to construction equipment and protection of existing structures. Observation visits to the site by the structural engineer shall not include inspection of the above items nor will the structural engineer be responsible for the contractor's means, methods, techniques, sequences for procedure of construction, or the safety precautions and programs incident there to.
- 6. Options are for the convenience of the contractor. He shall be responsible for all changes necessary if he chooses an option and shall coordinate all details. The cost of additional engineering work necessitated by selection of an option shall be borne by the contractor.
- 7. Any engineering design provided by others and submitted for review shall bear the seal and signature of an engineer registered in
- 8. Dimensions on the structural drawings are exact with the exception of masonry and sawn lumber dimensions which are nominal. Verify all dimensions with the architectural drawings.
- 9. Where required construction details are not shown or noted on these plans the contractor shall notify the engineer and the engineer shall provide sufficient details for the work to proceed.
- 10. Construction materials shall be spread out if placed on framed construction. Load shall not exceed the design live load per square foot.
- 11. Shop drawings shall be submitted to the architect for all structural items. The Engineers review is intended only as an aid to the contractor for obtaining correct shop drawings. Responsibility for correctness shall rest with the contractor. The contractor shall review all shop drawings prior to submittal. Items not in accordance with the contract documents shall be flagged upon his review. The shop drawings do not replace the original contract drawings. It is the contractors responsibility to make sure items are constructed to the original drawings. The adequacy of engineering designs and layout performed by others rests with the designing or submitting authority.

FOUNDATION:

1. The foundation design was based on the following assumed parameters:

- Passive 350 pcf

- Maximum allowable bearing pressure 2,000 psf Minimum required dead load pressure - 0 psf - Active 45 pcf Lateral soil pressure
- The actual soil condition shall be investigated and the above parameters shall be verified by a Soils Engineer, registered in the State of Colorado, prior to construction of the foundation. If there are any discrepancies with the above assumptions, then the foundation shall be re-designed.
- B. EXCAVATION
- 1. Contractor shall provide all necessary sheeting, shoring and bracing where required to properly and safely complete the work.
- 2. Avoid excessive wetting or drying of the foundation excavation during construction. Keep excavations reasonably free of water at all time and completely free of water during placement of concrete.
- 1. Footings shall bear on natural undisturbed soil below the frost depth required by the applicable building code. Any discrepancies with the footing elevations shown on the plans shall be brought to the attention of
- 2. Building shall be founded on continuous concrete spread footings placed on undisturbed natural soil.
- D. GENERAL REQUIREMENTS
- 1. Center foundations under columns and walls unless noted otherwise.
- 2. Provide foundation ventilation in crawl space areas as required by the I.R.C.
- 3. Backf II on walls with f II on both sides shall be compacted in equal lifts each side of wall. Walls backf Iled on one side only shall have all supporting slabs, permanent framing or temporary bracing in place prior to placement of the backf II (unless noted otherwise on plans).

CONCRETE:

- A. CONCRETE REQUIREMENTS
- Concrete has been designed and shall be constructed in accordance with the latest editions of the American Concrete Institute Building Code, ACI 318 and Specifications for Structural Concrete for Buildings, ACI 301. Provide hot or cold weather protection per ACI 305 & 306.
- 1. Concrete shall have a minimum compressive strength at 28 days of: Walls & footings 3000 psi 3000 psi
- 2. Provide Type II cement throughout with 4% air entrainment.
- Mechanically vibrate concrete.
- 4. No admixtures shall be used without approval by the engineer.
- 5. Addition of water to the batch material for insufficient slump is not permitted.
- 6. Do not place pipes, ducts or chases in the structural concrete without the approval of the engineer.
- 7. Thoroughly clean all case and construction joints prior to placing concrete in adjacent pour.
- 8. Concrete shall not be in contact with aluminum.

C. SLABS

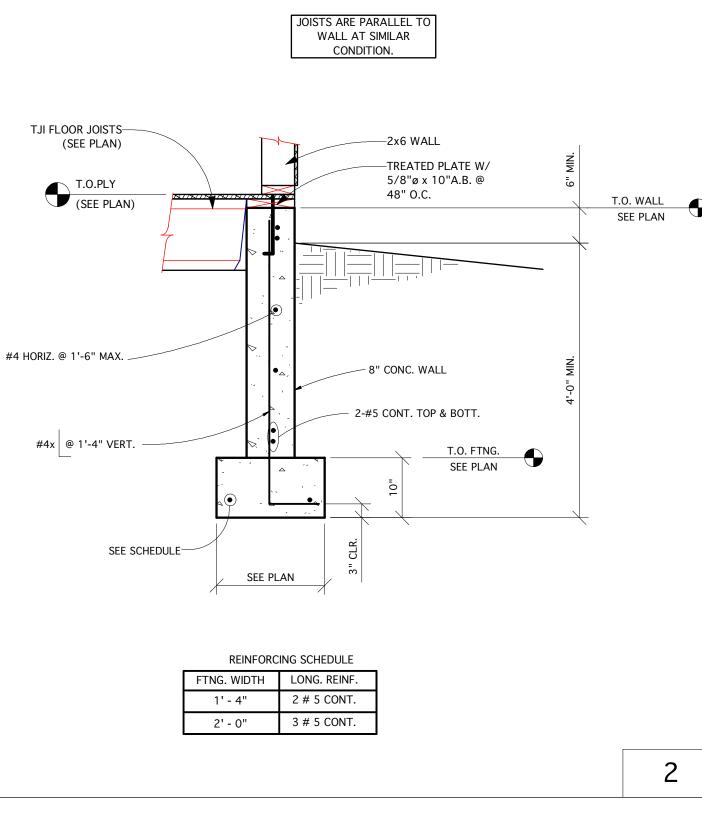
- Detailing, fabrication, and erection of reinforcing steel bars shall comply with the ACI Manual of Standard Practice for Detailing Reinforced Concrete Structures ACI 315.
- 1. Reinforcing bars shall be ASTM A615-Grade 60.
- 2. Welded wire fabric shall conform to ASTM A185. Furnish in flat sheets only.
- 3. Concrete protection for reinforcement: Cast against and permanently exposed to earth 3 in. Exposed to earth or weather............ #6-#18 bar....... 2 in.
- #3-#5 bar.....1 1/2 in. Not exposed to earth or weather..... slabs & walls.....3/4 in. beams & cols....1 1/2 in.
- 4. Reinforcing lap splices shall be a minimum of 36 bar diameters unless noted otherwise. Lap wire fabric reinforcement one full mesh plus 2" at sides and ends and wire together.
- 5. Splices in horizontal beams and walls shall occur at midspan for top bars and over supports for bottom
- 6. Discontinue one-half of horizontal steel across construction/control joints.
- 7. Provide corner bars of equal size and spacing around all corners.
- 8. Provide 2-#5 bars with a minimum of 2'-0" projection beyond the sides of all openings in walls, beams and slabs. Provide 1-#5 x 4'-0" diagonally at all re-entrant corners of slabs.
- 9. Provide accessories necessary to properly support reinforcing at the positions shown on the plans.
- 10. Reinforcing spacings are maximum on center and all reinforcing is continuous, unless noted otherwise.
- 1. Provide control joints at 12'-0" max. spacing or as shown on the plans.
- 2. Provide minimum 4" compacted gravel under f bor slab on grade. The material shall consist of minus 2-inch aggregate w/ less than 50% passing the No. 4 sieve and less than 2% passing the No. 200 sieve.

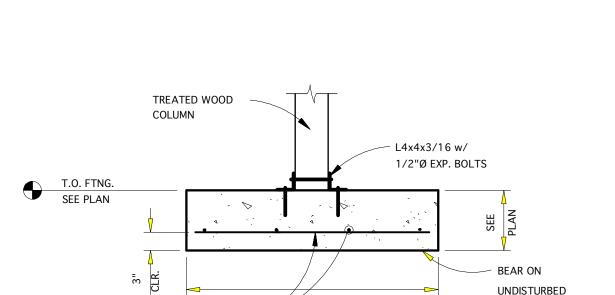
STRUCTURAL WOOD FRAMING:

- A. DIMENSIONAL LUMBER
- All wood members shall be as described in the 2001 edition of the "National design Specifications for Wood Construction" by the National Forest Products Association and the WWPAW.
- 1. Unless otherwise noted on the drawings, wood framing members shall be Douglas Fir/Larch, Grade #2 or better. Redwood members shall be construction grade.
- 2. Sizes shown are nominal, except when noted as RC (Rough Cut) they are full dimensions.
- 3. All wood members attached to concrete or masonry shall be redwood or pressure treated lumber.
- 4. Block all joists and rafters per the I.R.C.
- B. FASTENERS AND CONNECTORS Alternate fasteners and connectors to those listed below, which have I.C.B.O. approval, may be used with approval by the Engineer.
- 1. Connectors shown on the plans are manufactured by the Simpson Strong-Tie Company, Inc. Connectors by other manufactures shall be deemed as equivalent if their rated capacity is equal to or greater than that of the connector specified. Follow manufacturer's recommendations for installation of connectors.
- 2. All timber nailing and connections shall conform to the IRC and AITC.
- 3. Bolts for wood member connections shall be 3/4" diameter, ASTM A307 with a minimum 1 1/2" washer at each end (unless noted otherwise)
- 4. Self-drilling fasteners for the attachment to structural steel shall be "Teks" fasteners as manufactured
- C. PLYWOOD SHEATHING
- Plywood sheathing for roofs, f bors and exterior shear walls shall be APA Grade-Trademarked C-D, Exposure
- 1. Roof Diaphragms (240 plf) shall be 5/8", APA rated sheathing, 40/20 place with 8'-0" dimension perpendicular to the joist, rafter or truss span. End joists shall be staggered. Attach plywood at edges with 8d nails at 6" O.C. and at intermediate supports with 8d nails at 12" O.C.
- 2. Floor Diaphragms (250 plf) shall be 3/4", APA rated Sturdi-I-Floor, 24 O.C. place with 8'-0" dimension perpendicular to the joist span. End joints shall be staggered. Attach plywood with APA glue and at edges nail with 8d nails at 6" O.C. and at intermediate supports with 8d nails at 10" O.C. Sheathing shall be
- 3. Shear Wall Diaphragms (180 plf, 380 plf wind) shall be 1/2", APA rated sheathing, 24/0 with 8"-0" dimension vertical. End joints shall be staggered. Studs shall be 16" O.C. Attach plywood at edges with 10d nails at 4" O.C. and at intermediate supports with 8d nails at 6" O.C. Sheath all exterior walls full height with 1/2" plywood. At all corners, min. 4'-0" each direction, block plywood edges and nail as noted
- D. GLUE LAMINATED BEAMS Laminated member shall be designed and fabricated in accordance with "The Standard Specifications for the Design and Fabrication of Structural Glue Laminated Lumber" published by the AITC and the appropriate Lumber Producers' Association.
- 1. Use exterior glue for all exposed laminated members.
- 2. All members exposed to weather shall receive one coat of end sealer at trimmed ends. Seal all exposed surfaces with sealer coat as soon as practical after erection.
- 3. Appearance grade shall be in accordance with the architectural drawings.
- 4. If beam is cantilevered and the cantilever is greater than one-third of the back span, install beam
- 5. Allowable unit stresses for dry conditions of use required shall be as follows: Members stressed principally in bending such as beams & girders (AITC 24F-V4) unless otherwise
- noted on drawings. Fb=2400 psi Fv=165 psi Fc(perp)=650 psi E=1.8E6 psi Members stressed principally in compression or tension such as columns & truss members (AITC
- 24F-V8) Unless otherwise noted on drawings. Fc(parallel)=1650 psi E=1.6E6 ps
- Micro-laminated members shall be designed and fabricated in accordance with the "National Research Board" and the "National Design Specifications for Wood"
- 1. Allowable unit stresses for dry conditions of use required shall be as follows:
- Fb=2600 psi Fv=285 psi Fc(perp)= 750 psi E=1.8E6 psi 2. Install micro-laminated members in accordance with the manufacturer's recommendation.
- 3. Holes, cuts, or notches not previously approved by the engineer or the manufacturer shall not be made.

4. Micro-laminated lumber is marked on the drawings as "ML" and shall be 1 3/4" wide, unless noted

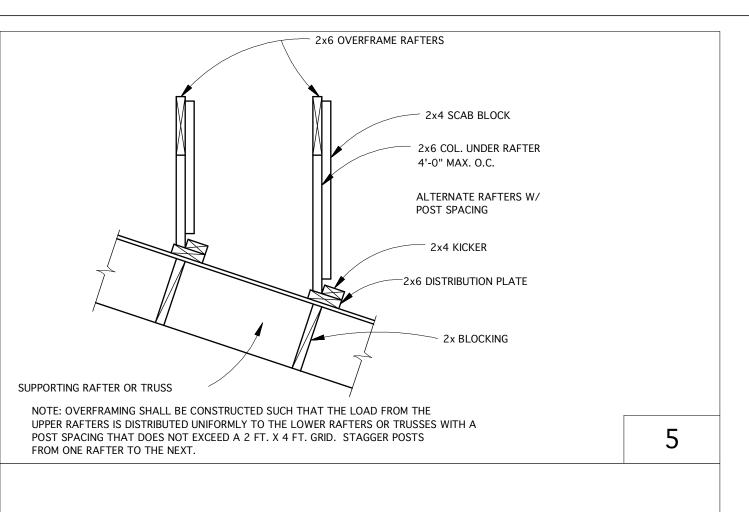
- otherwise.
- F. FABRICATED WOOD JOISTS (I-Series) I-series fabricated joists with structural wood fanges and plywood webs called for on the drawings (TJI/210, TJI/360, TJI/560) are as manufactured by the Trus-Joist Corp.. Joists by other manufacturers may be used if the depth and defection is equal to or less than that of the joist specified for the particular span and spacing and when authorized by the Architect/Engineer.
- 1. Fabricated wood joists shall be in accordance with the manufacturer's recommendations.
- 2. Supply all plates, blocking, bridging, bracing, stiffeners and other related items as recommended by the





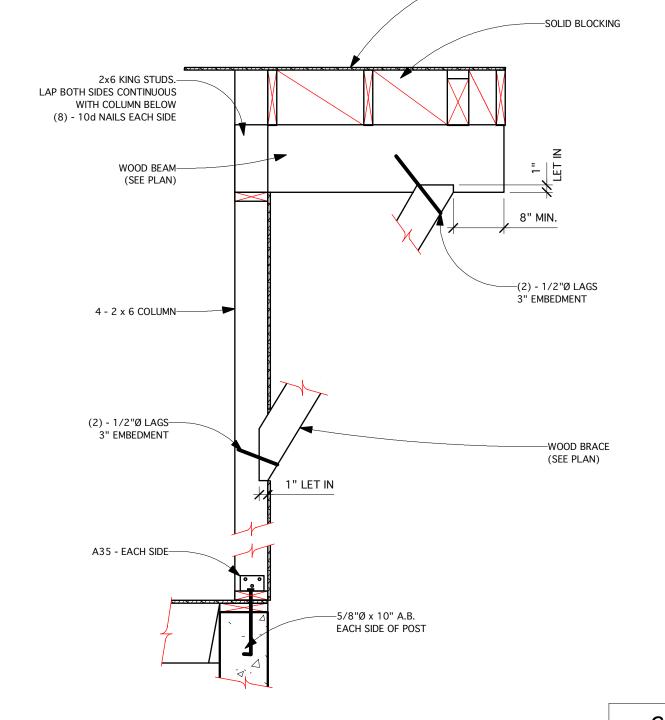
SEE PLAN FOR REINFORCING

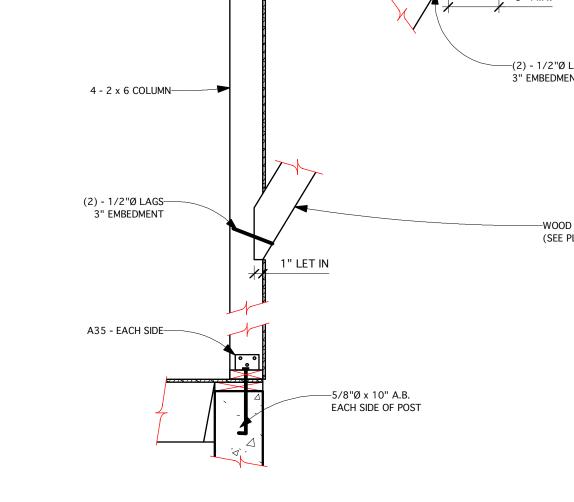
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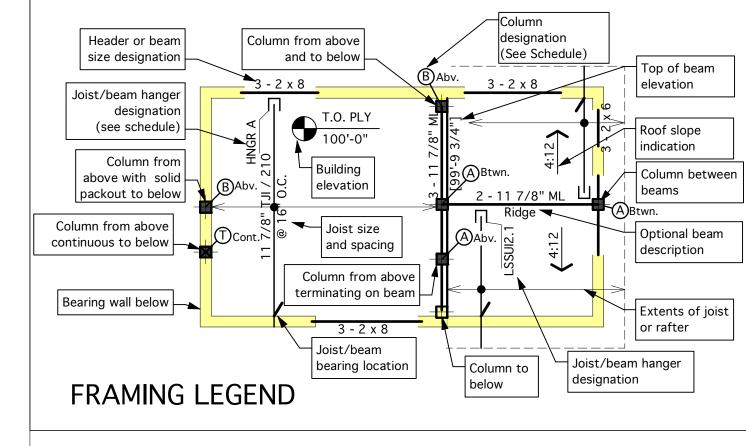


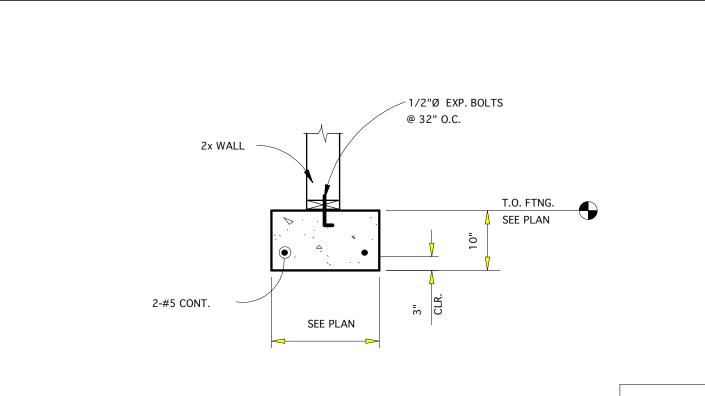
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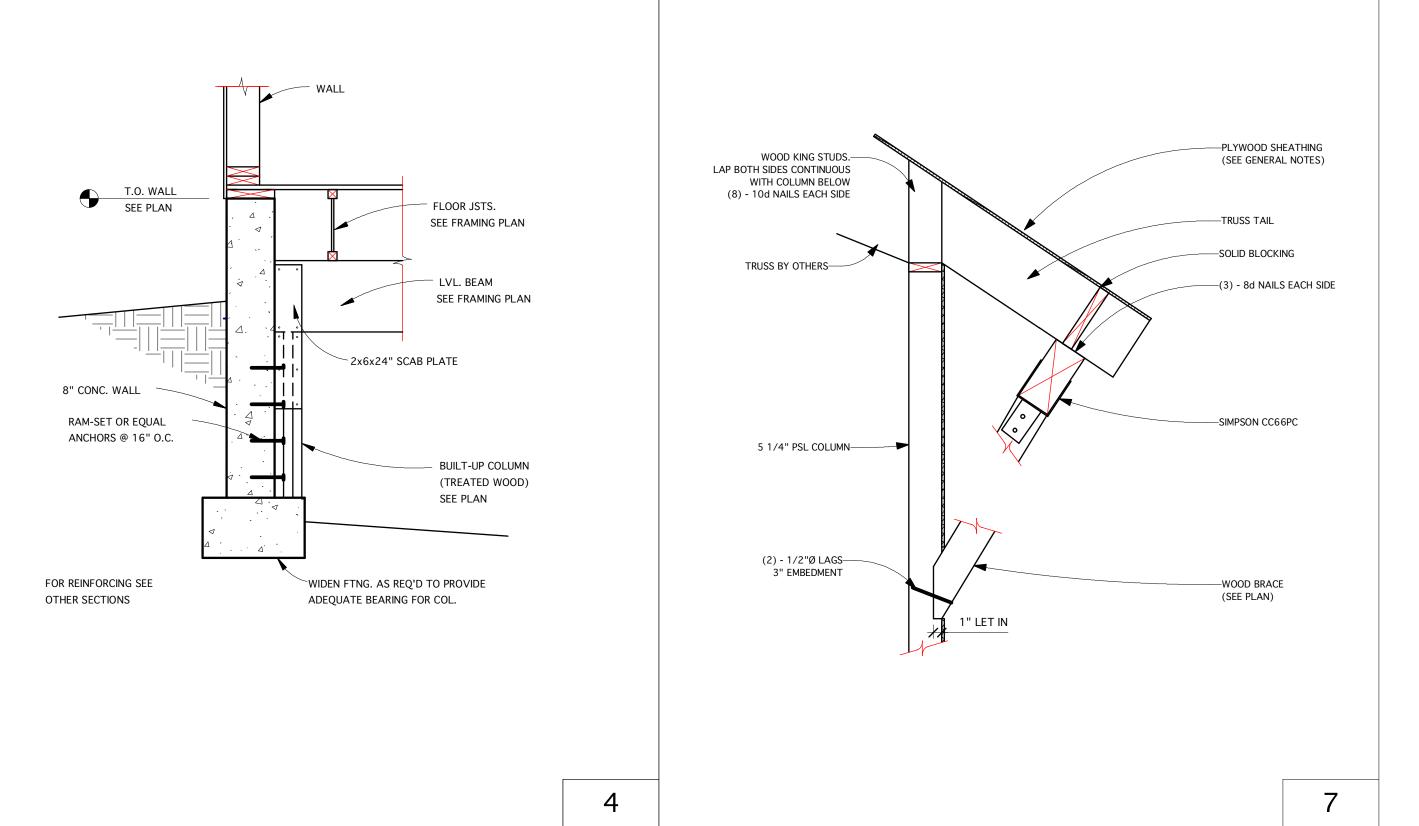
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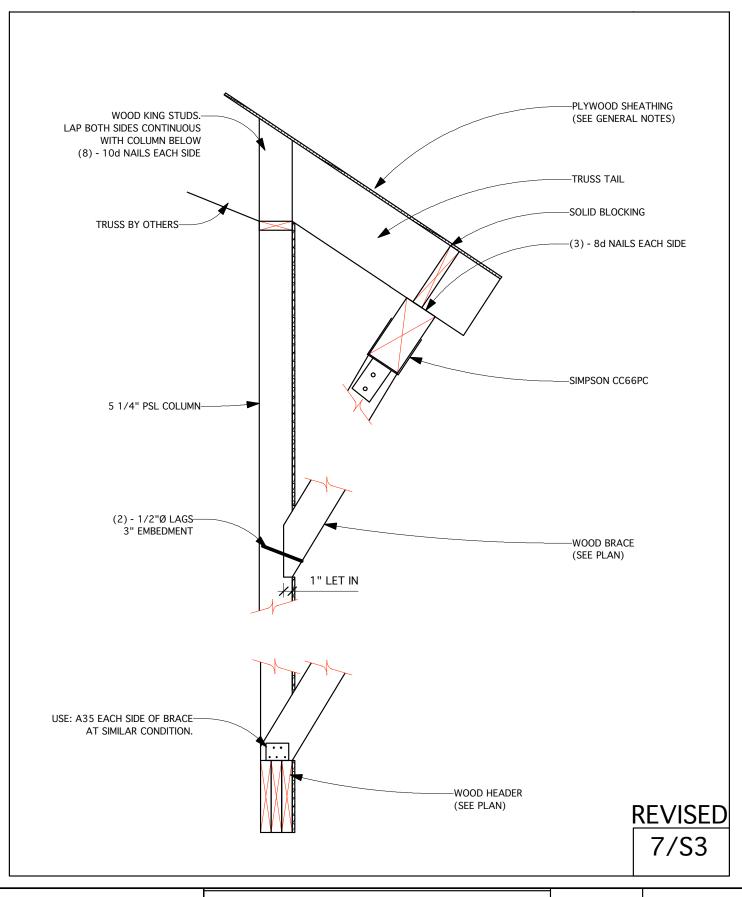




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SHEET NUMBER



ERNEST KOLLAR ENGINEERS, INC.

320 MAIN STREET, SUITE 203 Carbondale, CO 81623 (970) 963–3223 FAX (970) 963–9395

Aguilar 63 Sunrise LnEagle County, CO

Dwn by: HLB Chkd by:

Job #: 1013 Date: 4-5-11

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